MAYFAIR ELEMENTARY SCHOOL

3305 E. HOME AVE

PROJECT NAME: MAYFAIR ELEMENTARY SCHOOL

LOCATION: 3305 E. HOME AVE.
FRESNO, CA 93703

PROJECT DESCRIPTION: PROPOSED NEW CMU SITE WALL

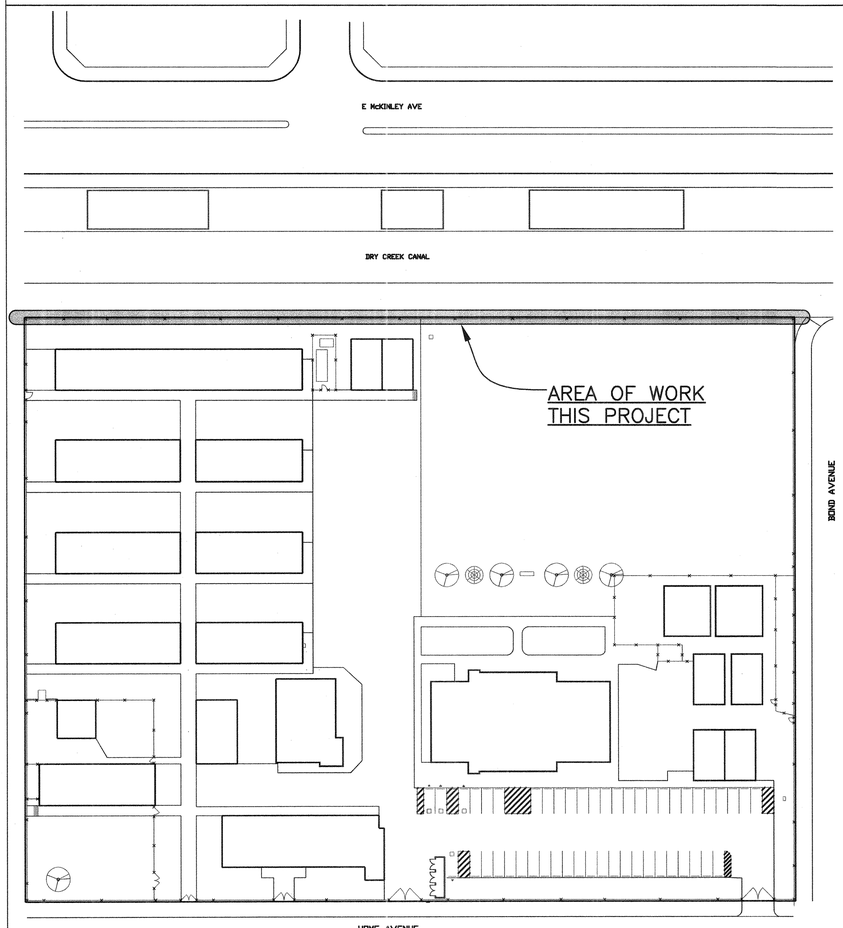
SHT. NO.	TITLE
C-1	COVER SHEET
S-1	GENERAL NOTES AND TYPICAL DETAILS
SP-1	DEMO PLAN AND SITE PLAN
SP-2	PROPOSED SITE PLAN
SP-2.1	PARTIAL SITE PLAN, PARTIAL ELEVATION, AND DETAILS
SP-2.2	PARTIAL ELEVATIONS

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
02 118215
AC TM PLS SS
DATE 2-12-20

DSA APP. 02-118215

ENGINEER	BWY
DRAWN BY	ER
DATE	03/06/2020
REVISIONS	

MAYFAIR ELEMENTARY SCHOOL
FRESNO UNIFIED SCHOOL DISTRICT
3305 E. HOME AVE, FRESNO, CA 93703



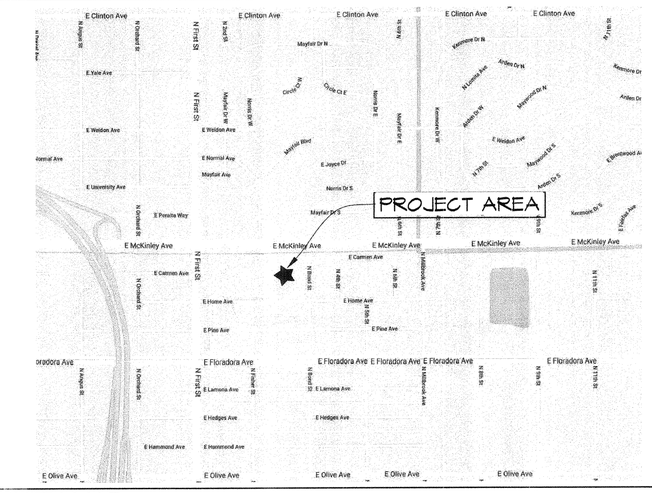
12 SITE PLAN

SCALE: N.T.S.

OWNER:	MAYFAIR ELEMENTARY SCHOOL 3305 E. HOME AVE. FRESNO, CA 93703 T: (559) 457-3140 CONTACT: KEVAN CARR (559) 351-5150	MAYFAIR ELEMENTARY Home of the Mustangs
STRUCTURAL:	BROOKS-RANSOM ASSOCIATES 7415 N. PALM AVE. STE. 100 FRESNO, CA 93711 T: (559) 449-8444 F: (559) 449-8404 CONTACT: ARTURO LOPEZ	BROOKS RANSOM ASSOCIATES STRUCTURAL ENGINEERS CIVIL ENGINEERS 7415 N. PALM AVE., SUITE 100 FRESNO, CALIFORNIA 93711

8 CONSULTANTS

SCALE: N.T.S.



9 VICINITY MAP

SCALE: N.T.S.

1 PROJECT INFORMATION

SCALE: N.T.S.

THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS IS TO CONSTRUCT THE CMU WALL IN ACCORDANCE WITH TITLES 19 AND 24, CALIFORNIA CODE OF REGULATIONS. SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE FINISHED WORK WILL NOT COMPLY WITH SAID TITLES 19 AND 24, CALIFORNIA CODE OF REGULATIONS, A CHANGE ORDER DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY OSHPD BEFORE PROCEEDING WITH THE WORK.

ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH CURRENT APPLICABLE CODES AND STANDARDS INCLUDING, BUT NOT LIMITED TO THE FOLLOWING:

CBC CALIFORNIA BUILDING CODE
C.C.R. TITLE 19, CHAPTER 1 (CSFM)
C.C.R. TITLE 24, PART 1, ADMIN. REGULATION, PART 2, CBC AND DSA/SS

CALIFORNIA CODE OF REGULATIONS (C.C.R.)

2019 CALIFORNIA BUILDING STANDARDS ADMINISTRATIVE CODE (CCR, TITLE 24, PART 1) EFFECTIVE JANUARY 1, 2020.

2019 CALIFORNIA BUILDING CODE (CBC), VOLUMES 1 AND 2 (CCR, TITLE 24, PART 2) BASED ON 2018 INTERNATIONAL BUILDING CODE.

ENFORCING AGENCIES:

1. DIVISION OF THE STATE ARCHITECT (DSA)

2 APPLICABLE CODES

SCALE: N.T.S.

CODE
2019 CALIFORNIA BUILDING CODE

RISK CATEGORY	TYPE III
WALL LOADS	
DEAD LOAD (C.M.U. WALL)	84 PSF
WIND LOADS	
BASIC WIND SPEED, V	105 MPH
WIND DIRECTIONALITY FACTOR, K_d	0.85
TOPOGRAPHIC FACTOR, K_{zt}	1.00
VELOCITY PRESSURE COEFFICIENT, K_z	0.85
GROUND ELEVATION FACTOR, K_e	1.00
VELOCITY PRESSURE, q_z	20.4 PSF
GUST EFFECT FACTOR, G	0.85
NET FORCE COEFFICIENT, C_f	1.3
EXPOSURE CATEGORY	C
DESIGN WIND FORCE, F	22.5 PSF
SEISMIC LOADING CRITERIA	
SEISMIC IMPORTANCE FACTOR,	1.25
MAPPED SPECTRAL ACCELERATION, MCE:	
a. S_s	0.604
b. S_1	0.232
SPECTRAL RESPONSE COEFFICIENT:	
a. F_a	1.317
b. F_v	2.186
MAXIMUM CONSIDERED EARTHQUAKE RESPONSE ACCELERATIONS:	
a. S_{MS}	0.795
b. S_{M1}	0.496
DESIGN SPECTRAL RESPONSE ACCELERATIONS:	
a. S_{DS}	0.530
b. S_{D1}	0.330
SEISMIC DESIGN CATEGORY	D
SEISMIC DESIGN FORCE, F_p	$0.318 * W_p$
SOILS	
ALLOWABLE BEARING PRESSURE	1500 PSF
LATERAL BEARING PRESSURE	100 PSF

3 DESIGN CRITERIA

SCALE: N.T.S.

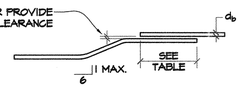
REGISTERED PROFESSIONAL ENGINEER
GAYLORD R. RANSOM
NO. S2388
EXPIRES 3-31-20
STRUCTURAL
STATE OF CALIFORNIA

BROOKS RANSOM ASSOCIATES
CIVIL ENGINEERS - STRUCTURAL ENGINEERS
7415 N. PALM, SUITE 100
FRESNO, CALIFORNIA 93711
TELEPHONE (559) 449-8444

PROJECT NUMBER
19590

SHEET NUMBER
C-1

LAP LENGTH								
BAR SIZE	#3	#4	#5	#6	#7	#8	#9	#10	#11
BOTTOM BAR	16"	24"	36"	48"	62"	72"	81"	91"	101"
TOP BAR	21"	32"	47"	56"	81"	94"	105"	118"	131"



NOTES:
1. SPLICES ARE SHOWN IN INCHES AND SHALL CONFORM TO CLASS "B" SPLICES AS PER A.C.I. 318-14, FOR 3000 PSI CONCRETE.
2. SPLICE LENGTHS ASSUME THE MODIFICATION FACTORS OF A.C.I. 318-14 SECTIONS 25.1.1 ARE 1.0. FOR OTHER CONDITIONS PROVIDE SPLICE LENGTHS IN ACCORDANCE WITH A.C.I. 318-14.
3. USE THE SPLICE LENGTH GIVEN FOR TOP BARS WHEN MORE THAN 12" OF CONCRETE IS CAST BELOW HORIZONTAL BARS IN THE MEMBER. USE THE SPLICE LENGTH GIVEN FOR BOTTOM BARS FOR ALL OTHER CONDITIONS.

10 LAP SPLICE SCHEDULE

FOR CONCRETE SCALE: N.T.S.

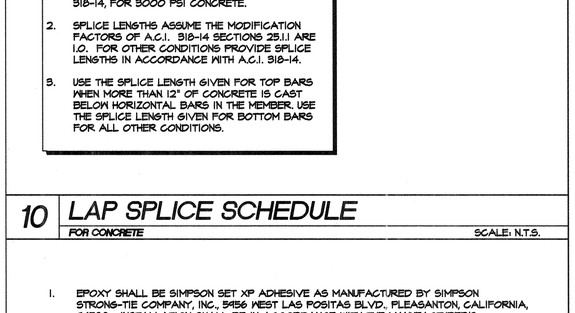
1. EPOXY SHALL BE SIMPSON SET XP ADHESIVE AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC., 5956 WEST LAS POSITAS BLVD. PLEASANTON, CALIFORNIA, 94588. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND I.C.C. REPORT NO. ESR-1772.
2. ALLOWABLE TENSION VALUES SHALL BE AS FOLLOWS:

BAR SIZE	BOLT SIZE	MIN. EMBEDMENT	MIN. EDGE DIST./ MIN. SPACING	TENSION LOADS (POUNDS)
#4	1/2"	4 1/4"	17"	1300
#5	5/8"	5"	20"	1645
#6	3/4"	6 3/4"	27"	3130

3. PLACEMENT GUIDELINES FOR ABOVE VALUES IN ITEM 2 REQUIRE THE FOLLOWING CONDITIONS:
   - TABLE VALUES ARE BASED ON f'_m = 1500 PSI/GROUT = 2000 PSI
   - MAXIMUM LONG TERM TEMPS OF 110° F OR LESS
   - DRILL BIT DIAMETER EQUALS d_{bar} + 1/8"
   - BOLT/BAR INSTALLATION @ FACE OF FULLY GROUTED CMU
   - HOLES SHALL BE CLEANED ACCORDING TO ICC REPORT NO. ESR-1772
4. WHEN INSTALLING EPOXIED REBAR/BOLTS IN EXISTING FULLY GROUTED CMU MASONRY, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE EXISTING REINFORCING BARS. MAINTAIN A MINIMUM CLEARANCE OF ONE-INCH BETWEEN THE EXISTING REINFORCEMENT AND THE EPOXIED REBAR/BOLT.
5. ANY REBAR/BOLTS SHOWN ON THE APPROVED PLANS AS BEING EPOXIED REQUIRES SPECIAL INSPECTION IN ACCORDANCE WITH SECTION 4.4 IN THE I.C.C. REPORT. SPECIAL INSPECTION SHALL BE BY AN APPROVED TESTING AND INSPECTION AGENCY. ANY ITEMS THAT REQUIRE EPOXY BUT ARE NOT SPECIFICALLY SHOWN AS BEING EPOXIED ON THE APPROVED PLANS MUST BE APPROVED BY THE STRUCTURAL ENGINEER AND D.S.A./O.S.H.P.D. PRIOR TO INSTALLATION.

11 SIMPSON SET XP EPOXY NOTES

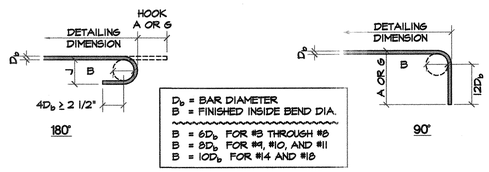
IN FULLY GROUTED CMU SCALE: N.T.S.



NOTES:
1. THE MAX. ALLOWABLE FTG. RISE = 18" & SLOPE = 1:2 U.N.O. IN THE SOILS REPORT.
2. FOR FOOTINGS WITHOUT EITHER BOTTOM OR TOP REINFORCEMENT, PROVIDE #5 @ 12" O.C. (2 MIN), TOP & BOTTOM IN AREA OF STEPPED FOOTING.
3. PROVIDE A MIN. LAP SPLICE FOR ALL DIAGONAL REINF. INTO CONT. FOOTING BEYOND STEPPED FOOTING AREA. SEE TYP. LAP SPLICE DTL. FOR ADDL. INFO.
4. PROVIDE STIRRUPS IN SLOPED FOOTING SECTION IF STIRRUPS ARE REQUIRED IN TYPICAL CONT. FOOTING SECTION. SEE PLAN FOR ADDL. INFORMATION.

12 STEPPED FOOTING

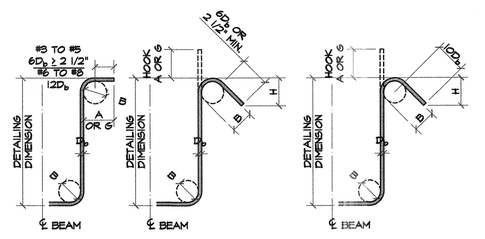
- SCALE: N.T.S.



D_b = BAR DIAMETER
B = FINISHED INSIDE BEND DIA.

B = $6D_b$ FOR #3 THROUGH #8
B = $8D_b$ FOR #9, #10, AND #11
B = $10D_b$ FOR #14 AND #18

DIMENSION OF STD. 180° HOOKS, ALL GRADES				DIMENSION OF STD. 90° HOOKS, ALL GRADES	
BAR SIZE	A OR G	J	B	A OR G	B
#3	5"	3"	2 1/4"	6"	2 1/4"
#4	6"	4"	3	8"	3
#5	7"	5"	3 3/4"	10"	3 3/4"
#6	8"	6"	4 1/2"	1'-0"	4 1/2"
#7	10"	7"	5 1/4"	1'-2"	5 1/4"
#8	11"	8"	6"	1'-4"	6"
#9	1'-3"	11 3/4"	9 1/2"	1'-7"	9 1/2"
#10	1'-5"	1'-1 1/4"	10 3/4"	1'-10"	10 3/4"
#11	1'-7"	1'-2 3/4"	12"	2'-0"	12"
#14	2'-3"	1'-4 3/4"	18 1/4"	2'-7"	18 1/4"
#18	3'-0"	2'-4 1/2"	24"	3'-5"	24"



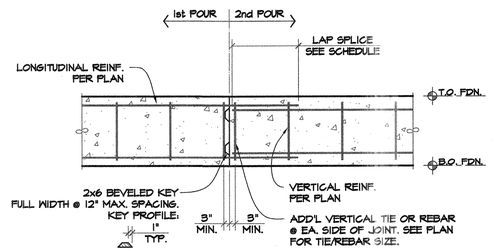
STIRRUP HOOKS (TIE BENDS SIMILAR)

SEISMIC STIRRUP/TIE

BAR SIZE	B	90°	135°		135° SEISMIC HOOK			
		A OR G	A OR G	APPROX. H	SIZE	B	A OR G	APPROX. H
#3	1 1/2"	4"	4"	2 1/2"	#3	1 1/2"	4 1/4"	3"
#4	2"	4 1/2"	4 1/2"	3"	#4	2"	4 1/2"	3"
#5	2 1/2"	6"	5 1/2"	3 3/4"	#5	2 1/2"	5 1/2"	3 3/4"
#6	4 1/2"	1'-0"	8"	4 1/2"	#6	4 1/2"	8"	4 1/2"
#7	5 1/4"	1'-2"	9"	5 1/4"	#7	5 1/4"	9"	5 1/4"
#8	6"	1'-4"	10 1/2"	6"	#8	6"	10 1/2"	6"

8 STANDARD REBAR HOOKS

- SCALE: N.T.S.



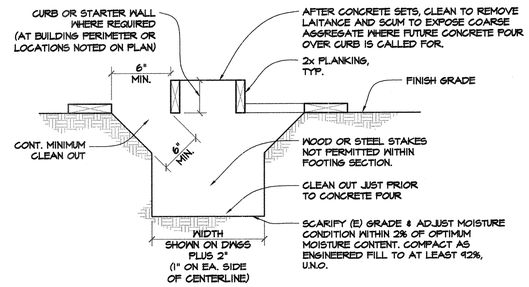
9 CONCRETE FOOTING JOINT

- SCALE: N.T.S.

1. CONCRETE MASONRY UNITS SHALL BE NORMAL WEIGHT, TYPE "N" AS SET FORTH IN A.S.T.M. C90. THE FOLLOWING MINIMUM STRENGTH FOR TYPE I, MOISTURE CONTROLLED UNITS SHALL BE:
   - 8" CMU f'_m = 2000 PSI
2. SEE ARCHITECTURAL DRAWINGS FOR COLOR & SURFACE TREATMENT OF EXPOSED CMU UNITS.
3. MASONRY, EXCEPT MASONRY VENEER, SHALL BE CONSTRUCTED WITHIN THE TOLERANCES SPECIFIED IN A.C.I. 530.1/A.S.C.E. 6/T.M.S. 602.
4. MASONRY UNITS SHALL BE LAID IN RUNNING BOND, U.N.O.
5. MORTAR SHALL CONFORM TO A.S.T.M. C270 AND BE TYPE "S" WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS WITH MORTAR PROPORTIONS PER A.S.T.M. C270 TABLE 1. LIME SHALL BE THE LAST MATERIAL ADDED TO THE MIXER.
6. GROUT SHALL CONFORM TO A.S.T.M. C476 OR C.B.C. TABLE 2108A.1.2, AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS. THE WATER CONTENT EXPRESSED ON A SATURATED SURFACE-DAY BASIS SHALL NOT EXCEED 0.7 TIMES THE WEIGHT OF CEMENT. AGGREGATE FOR GROUT SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN A.S.T.M. C404.
7. GROUT SHOULD BE PUDDLED OR TAMPED WITH A 5/8" ROD OR A 1"x2" STICK AS IT IS PLACED.
8. ALL CELLS SHALL BE GROUTED SOLID BY EITHER A LOW LIFT (4'-0" MAX.) OR HIGH-LIFT GROUTING METHOD. LOW-LIFT GROUTED CONSTRUCTION MUST COMPLY WITH THE REQUIREMENTS OF C.B.C. 2104A.1.3.1.1.1.1 HIGH-LIFT GROUTED CONSTRUCTION MUST HAVE THE APPROVAL OF THE STRUCTURAL ENGINEER AND D.S.A./O.S.H.P.D., AND MUST COMPLY WITH THE REQUIREMENTS OF C.B.C. 2104A.1.3.1.1.2. IN EITHER GROUTING METHOD, THE GENERAL CONDITIONS OF C.B.C. 2104A.1.3 ALSO APPLY.
9. GROUTING OF BEAMS OVER OPENINGS SHALL BE DONE IN CONTINUOUS OPERATION WITH A MINIMUM 12" SEAT AT EACH END UNLESS PLACED IN TOTAL WITH WALL GROUTING. ALL WALL OPENINGS SHALL BE SHORED FOR A MINIMUM OF 28 DAYS AFTER COMPLETION OF GROUTING, U.N.O.
10. ALL REINFORCING SHALL CONFORM TO A.S.T.M. A615 AND SHALL BE GRADE 40 FOR #3 BAR, GRADE 60 FOR #4 BAR AND LARGER.
11. ALL WELDING OF REINFORCING STEEL SHALL BE WITH LOW HYDROGEN ELECTRODES U.N.O. WELDING OF REINFORCING SHALL BE ALLOWED ONLY WHERE DETAILED ON DRAWINGS. ALL WELDING SHALL BE DONE IN ACCORDANCE WITH AMERICAN WELDING SOCIETY SPECIFICATIONS AWS D1.4. WELDING SHALL NOT BE DONE WITHIN TWO BAR DIAMETERS OF ANY BENT PORTION OF A BAR WHICH HAS BEEN BENT COLD. WELDING OF CROSSING BARS SHALL NOT BE PERMITTED FOR ASSEMBLY OF REINFORCEMENT UNLESS AUTHORIZED BY THE STRUCTURAL ENGINEER OF RECORD. A.S.T.M. A706 REINFORCING SHALL BE USED FOR ALL REINFORCING THAT IS BEING WELDED.
12. VERTICAL REINFORCING SHALL BE CENTERED IN WALL, U.N.O., AND PLACED IN OPEN END UNITS BEFORE BLOCK WORK BEGINS. VERTICAL REINFORCING SHALL BE HELD IN POSITION AT TOP, BOTTOM AND AT INTERVALS NOT EXCEEDING 192 BAR DIAMETERS.
13. HORIZONTAL REINFORCING SHALL BE LAID IN BOND BEAM UNITS AND SECURELY WIRED TO THE VERTICAL REINFORCING.
14. DOWELS IN ALL FOUNDATIONS SHALL HAVE THE SAME LOCATION, SIZE, AND SPACING AS THE VERTICAL MASONRY REINFORCING.
15. PROVIDE CLEANOUT OPENINGS FOR ALL WALLS AT THE BOTTOM OF EACH POUR IN ACCORDANCE WITH C.B.C. 2019 REQUIREMENTS FOR HIGH LIFT GROUTING.
16. ANCHOR BOLTS MUST BE SET WITH TEMPLATES AND HELD IN PLACE PRIOR TO GROUTING. PROVIDE AT LEAST 1" OF GROUT BETWEEN THE HEAD OF THE ANCHOR BOLT AND THE INSIDE FACE OF MASONRY SHELL.
17. ALL ANCHOR BOLTS SHALL BE HEX-HEADED U.N.O.
18. PLACEMENT OF REINFORCEMENT REQUIREMENTS:
   - THE CLEAR DISTANCE BETWEEN PARALLEL BARS SHALL NOT BE LESS THAN THE NOMINAL DIAMETER OF THE BAR, NOR LESS THAN 1".
   - IN COLUMNS AND PILASTERS, THE CLEAR DISTANCE BETWEEN VERTICAL BARS SHALL NOT BE LESS THAN 1.5 TIMES THE NOMINAL BAR DIAMETER, NOR LESS THAN 1 1/2".
   - REINFORCEMENT EMBEDDED IN GROUT SHALL HAVE A THICKNESS OF GROUT BETWEEN THE REINFORCEMENT AND MASONRY UNIT NOT LESS THAN 1/2".
19. REINFORCING BARS SHALL HAVE A MASONRY COVER NOT LESS THAN THE FOLLOWING:
   - MASONRY FACE EXPOSED TO EARTH OR WEATHER: 2" FOR #6 & GREATER BARS, 1 1/2" FOR #5 & SMALLER BARS.
   - MASONRY NOT EXPOSED TO EARTH OR WEATHER: 1 1/2" FOR ALL BAR SIZES.

5 MASONRY NOTES

- SCALE: N.T.S.



NOTES:
1. FOUNDATION CONCRETE MAY BE PLACED DIRECTLY INTO NEAT EXCAVATIONS PROVIDED THAT FOUNDATION TRENCH WALLS ARE STABLE AS DETERMINED BY THE ARCHITECT, OR STRUCTURAL ENGINEER, SUBJECT TO THE APPROVAL OF THE D.S.A./O.S.H.P.D. IN SUCH CASE, THE MINIMUM FORM WORK SHOWN ABOVE IS MANDATORY TO INSURE CLEAN EXCAVATIONS IMMEDIATELY PRIOR TO, AND DURING, THE PLACING OF CONCRETE.
2. FORMWORK NOT PERMITTED BELOW GRADE UNLESS FULLY FORMED.

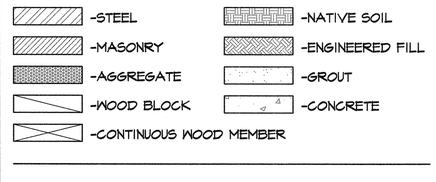
6 REQUIRED CONT. FTG. FORM WORK

- SCALE: N.T.S.

1. FOUNDATIONS SHALL BEAR ON ENGINEERED FILL OR NATIVE SOIL A MINIMUM OF 18" BELOW ADJACENT GRADE OR FINISHED GRADE, U.N.O.
2. MAXIMUM SIZE AGGREGATE SHALL BE AS FOLLOWS:
   - FOOTINGS: 1 1/2"
3. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE AS FOLLOWS:
   - 3000 PSI NORMAL WEIGHT FOOTINGS
4. MAXIMUM WATER CEMENT RATIOS SHALL BE AS FOLLOWS:
   - 0.58 FOOTINGS
5. THE FOLLOWING ARE MINIMUM CONCRETE COVER DIMENSIONS PER ACI 318-14 SECTION 20.6.1. THEY ARE FROM FACE OF REINFORCING STEEL TO FACE OF CONCRETE.
   - CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
   - CONCRETE EXPOSED TO EARTH OR WEATHER: (NO. 6 THROUGH NO. 18) 2"; (NO. 5 AND SMALLER) 1 1/2"
   - PLACE REINF. AT MID-THICKNESS FOR SLABS ON GRADE.
6. CONSTRUCTION LOADS SHALL NOT BE PLACED ON NEW CONCRETE CONSTRUCTION FOR AT LEAST 7 DAYS AFTER CONCRETE PLACEMENT OR WITH APPROVAL BY ENGINEER.
7. ALL SPLICES IN CONTINUOUS REINFORCEMENT USED IN WALLS, FOOTINGS, ETC. SHALL HAVE A MINIMUM LAP AS DESCRIBED IN THE TYPICAL LAP SPLICE SCHEDULE 10/- DETAIL. SPLICES IN ADJACENT BARS SHALL NOT BE LESS THAN 4'-0" APART. VERTICAL WALL BARS SHALL BE SPLICED AT OR NEAR FLOOR LINES. BARS MAY BE WIRED TOGETHER AT SPLICES OR LAPPED EXCEPT FOR TOP REINF. OF BEAM AND SLABS, OR WHERE SPECIFICALLY DETAILED TO BE SEPARATED.
8. ALL REINFORCEMENT CROSSING CONSTRUCTION JOINTS SHALL BE CONTINUOUS, OR SHALL BE MADE EFFECTIVELY CONTINUOUS BY USE OF FULLY DEVELOPED LAP SPLICES, DOWELS (WITH LAPPED SPLICES) OR APPROVED COUPLERS.
9. HORIZONTAL CONSTRUCTION JOINTS SHALL HAVE ENTIRE SURFACE REMOVED TO EXPOSE CLEAN AGGREGATE SOLIDLY EMBEDDED.
10. CONCRETE SHALL NOT BE DROPPED THROUGH REINF. STEEL (AS IN WALL) SO AS TO CAUSE SEGREGATION OF AGGREGATES. IN SUCH CASES, HOPPERS AND VERTICAL CHUTES OR TRUNKS SHALL BE USED. CHUTES OR TRUNKS SHALL BE OF VARIABLE LENGTHS SO THAT FREE UNCONFINED FALL OF CONCRETE SHALL NOT EXCEED FIVE (5) FEET AND A SUFFICIENT NUMBER OF CHUTES AND TRUNKS SHALL BE USED TO ENSURE THE CONCRETE REMAINS LEVEL AT ALL TIMES.
11. ALL STEEL COLUMN BASE PLATES AND STEEL BEAMS BEARING ON CONCRETE SHALL BEAR UPON 1 1/2" OF NON-SHRINK, 5,000 PSI MIN, GROUT PADS AND LEVELING NUTS, U.N.O.
12. CONTRACTOR SHALL SUBMIT PROPOSED POUR SCHEDULE FOR ENGINEER'S APPROVAL PRIOR TO THE FORMING OR POURING OF ANY CONCRETE WORK.
13. PROVIDE 3/4" CHAMFER AT EXPOSED EDGES OF CONCRETE BEAMS, COLUMNS AND WALLS UNLESS NOTED OTHERWISE.
14. THE CONTRACTOR SHALL FURNISH AND INSTALL 1/2" PRE-MOLDED EXPANSION JOINTS IN ALL EXTERIOR WALKS AND SLABS AS INDICATED ON DRAWINGS, BUT IN NO CASE MORE THAN 24'-0" O.C.
15. ALL REINFORCING SHALL CONFORM TO A.S.T.M. A615 AND SHALL BE GRADE 40 FOR #3, GRADE 60 FOR #4 AND LARGER.
16. ALL WELDING OF REINFORCING STEEL SHALL BE WITH LOW HYDROGEN ELECTRODES U.N.O. WELDING OF REINFORCING SHALL BE ALLOWED ONLY WHERE DETAILED ON DRAWINGS. ALL WELDING SHALL BE DONE IN ACCORDANCE WITH AMERICAN WELDING SOCIETY SPECIFICATIONS AWS D1.4. WELDING SHALL NOT BE DONE WITHIN TWO BAR DIAMETERS OF ANY BENT PORTION OF A BAR WHICH HAS BEEN BENT COLD. WELDING OF CROSSING BARS SHALL NOT BE PERMITTED FOR ASSEMBLY OF REINFORCEMENT UNLESS AUTHORIZED BY THE STRUCTURAL ENGINEER OF RECORD. A.S.T.M. A706 REINFORCING SHALL BE USED FOR ALL REINFORCING THAT IS BEING WELDED.

2 FOUNDATION AND CONC. NOTES

- SCALE: N.T.S.



3 LEGEND AND SYMBOLS

- SCALE: N.T.S.

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
02 118215
AC TM FLS SS KP
DATE 2-10-20

DSA APP. 02-118215

ENGINEER	BWY
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MAYFAIR ELEMENTARY SCHOOL
FRESNO UNIFIED SCHOOL DISTRICT
3305 E. HOME AVE., FRESNO, CA 93703

REGISTERED PROFESSIONAL ENGINEER
GAYLORD R. RANSOM
NO. S2386
EXPIRES 3-31-20
STRUCTURAL
STATE OF CALIFORNIA

BROOKS RANSOM ASSOCIATES
CIVIL ENGINEERS - STRUCTURAL ENGINEERS
7415 N. PALM, SUITE 100
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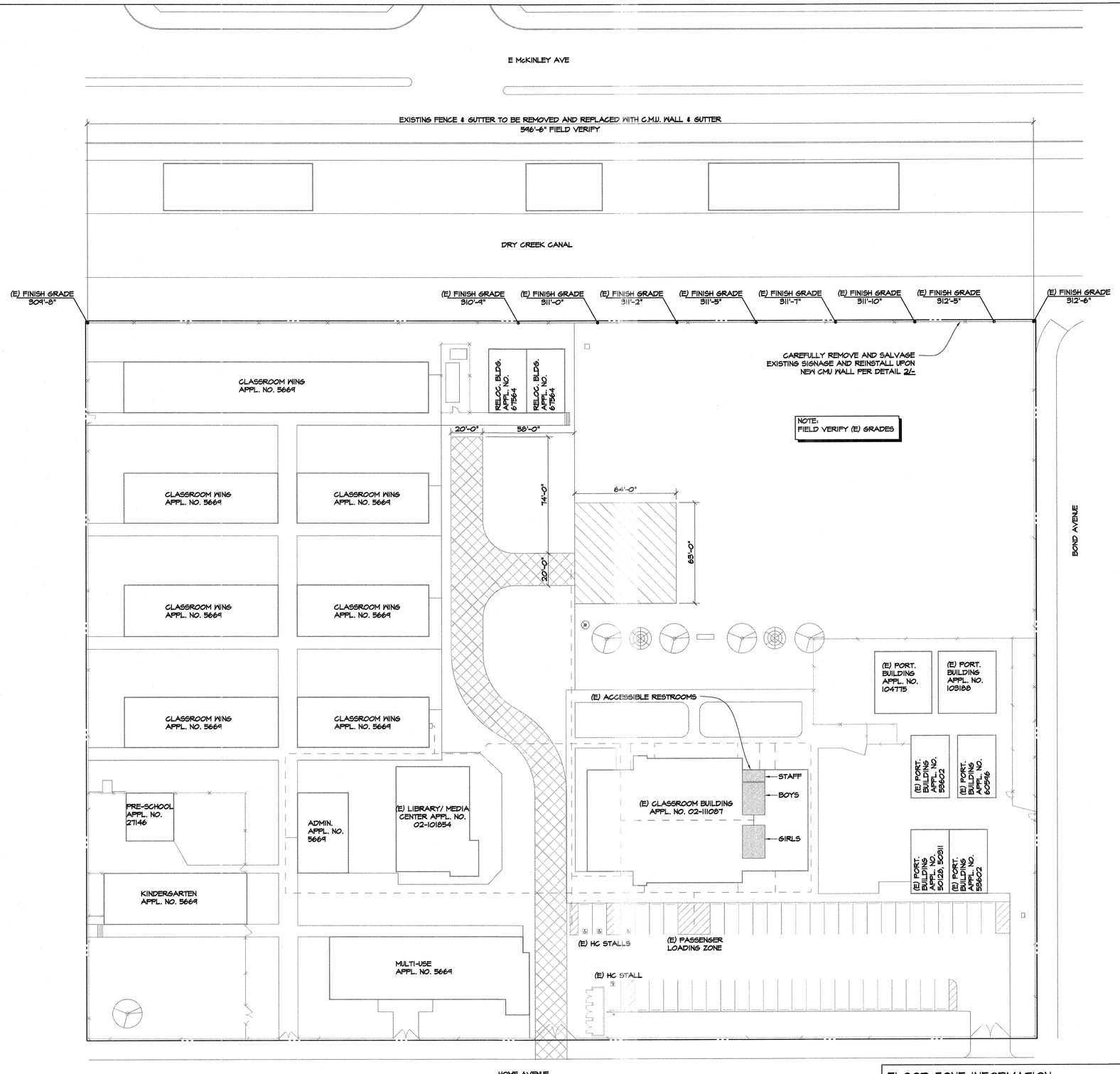
TELEPHONE (559) 449-8444

PROJECT NUMBER 19590

SHEET NUMBER S-1



12 DEMO PLAN AND SITE PLAN

SCALE: 1/32"=1'

FLOOD ZONE INFORMATION

THE PROJECT SITE IS IDENTIFIED ON FEMA FLOOD MAP 06019C1570H.FRIM PANEL 1570AS ZONE X. ZONE X IS AN AREA DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOOD PLAIN.

PROPERTY LINE

(E) CHAINLINK FENCE

(E) ACCESSIBLE PATH OF TRAVEL. ENTIRE CAMPUS IS ACCESSIBLE. ALL EXTERIOR WALKS AND DOORS COMPLY WITH THE REQUIREMENTS FOR THE ACCESSIBLE P.O.T.

(E) FIRE ACCESS LANE APPL. NO. 02-111087

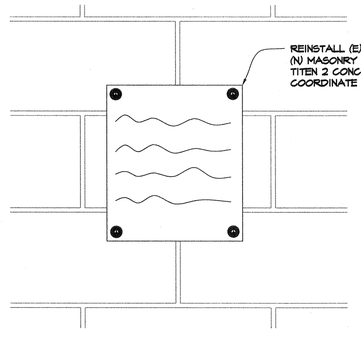
(E) SAFE DISPERSAL AREA APPL. NO. 02-111087

(E) FIRE HYDRANT

(E) TREE TO REMAIN

1 LEGEND

SCALE: N.T.S.



2 SIGN ANCHORAGE DETAIL

SCALE: 2" = 1'-0"

3 NOT USED

SCALE: N.T.S.

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
02 118215
AC TM FLS — SS
DATE 3-18-20

DSA APP. 02-118215

ENGINEER	BWY
DRAWN BY	ER
DATE	03/06/2020
REVISIONS	

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PROJECT NUMBER
19590

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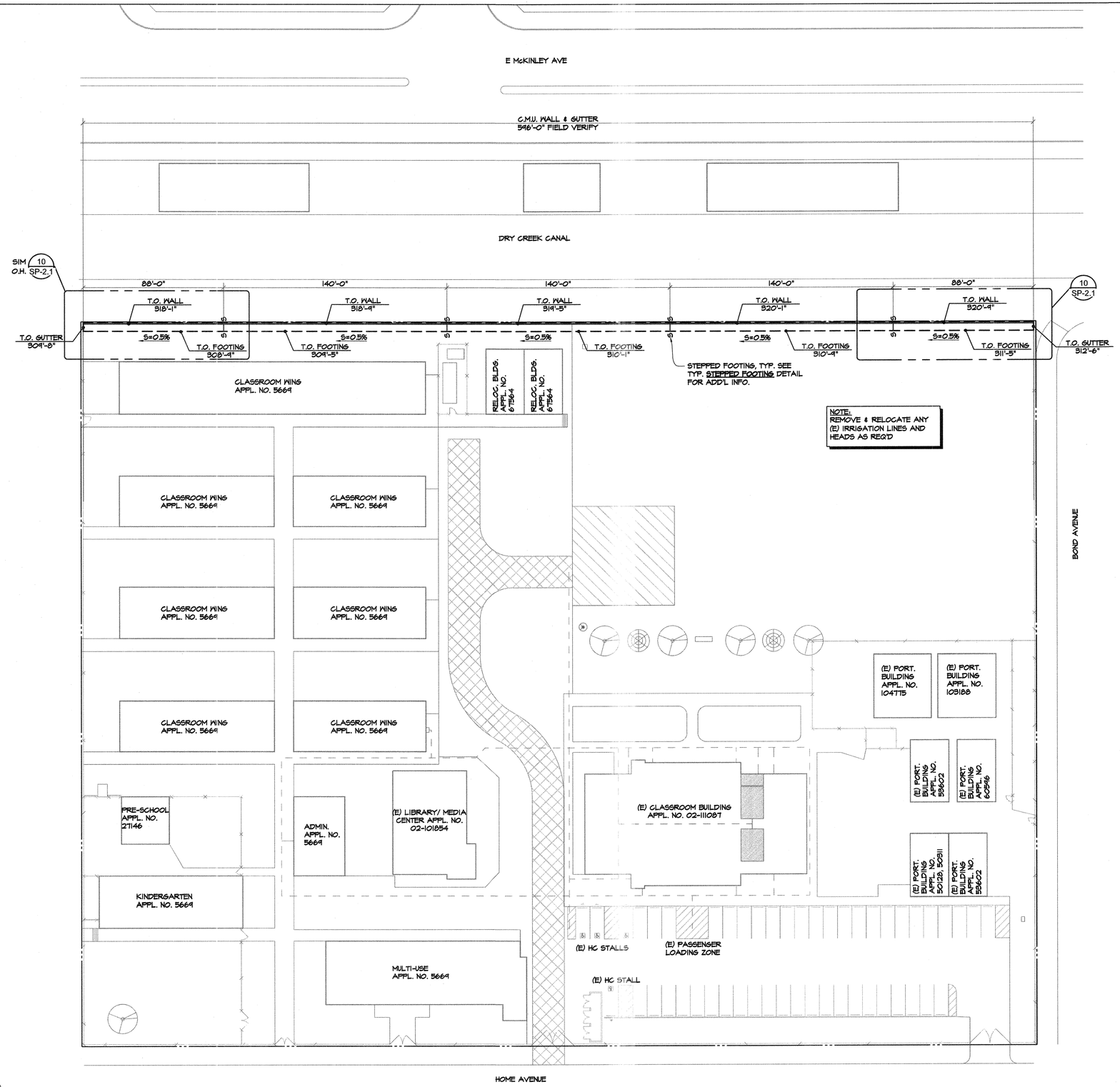
SP-1



12 PROPOSED SITE PLAN
- SCALE: 1/32"=1'

1 NOT USED
- SCALE: N.T.S.

2 NOT USED
- SCALE: N.T.S.

3 NOT USED
- SCALE: N.T.S.

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
02 118215
AC. TM FIN. SS
DATE 2-10-21

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ENGINEER	BWY
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DATE	03/06/2020
REVISIONS	

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PROJECT NUMBER
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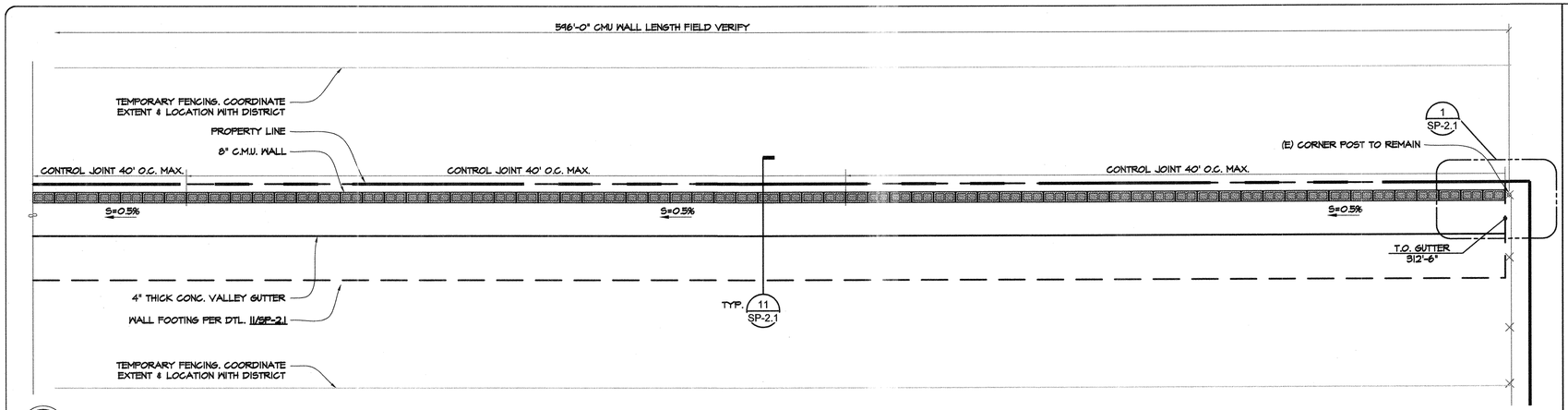
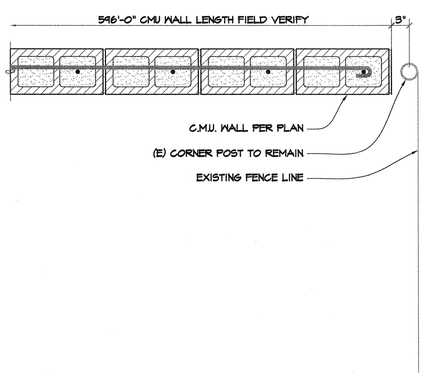
SHEET NUMBER
SP-2



10 PARTIAL SITE PLAN

SCALE: 1/4" = 1'



1 DETAIL

SCALE: 1" = 1'

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DIV. OF THE STATE ARCHITECT
02 118215
AC TM FLS SS
DATE 3-2-20-21

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ENGINEER	BWY
DRAWN BY	ER
DATE	03/06/2020
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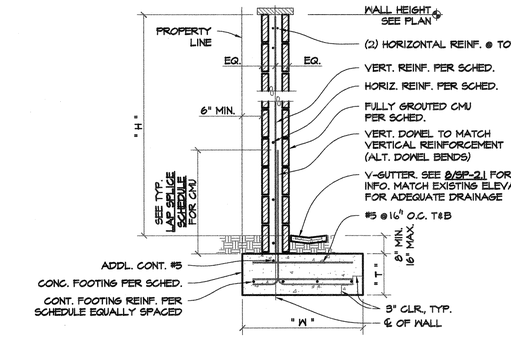
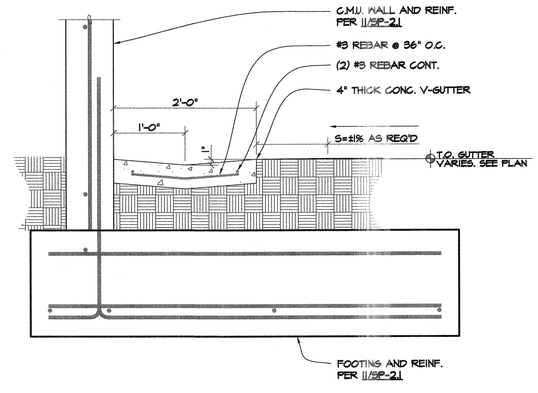


WALL SCHEDULE						
H	BLOCK THICKNESS	W	T	VERTICAL REINF.	HORIZ. REINF.	FOOTING REINF.
8'-0" MAX.	8"	5'-4"	1'-6"	#5 @ 16" O.C.	#5 @ 24" O.C.	(9) #5

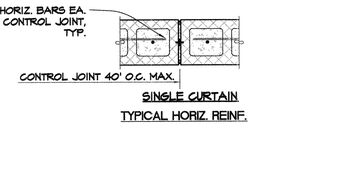
11 FREE STANDING MASONRY WALL

ON PROPERTY LINE — SCALE: N.T.S.



8 VALLEY GUTTER

SCALE: 1" = 1'



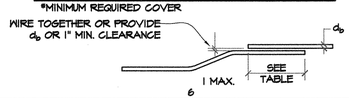
SINGLE CURTAIN
TYPICAL HORIZ. REINF.

5 CMU WALL CONTROL JOINT

SCALE: N.T.S.

LAP LENGTH							
BAR SIZE	#3	#4	#5	#6	7#	#8	#9
1 1/2 INCHES*	18"	36"	45"	54"	63"	72"	82"

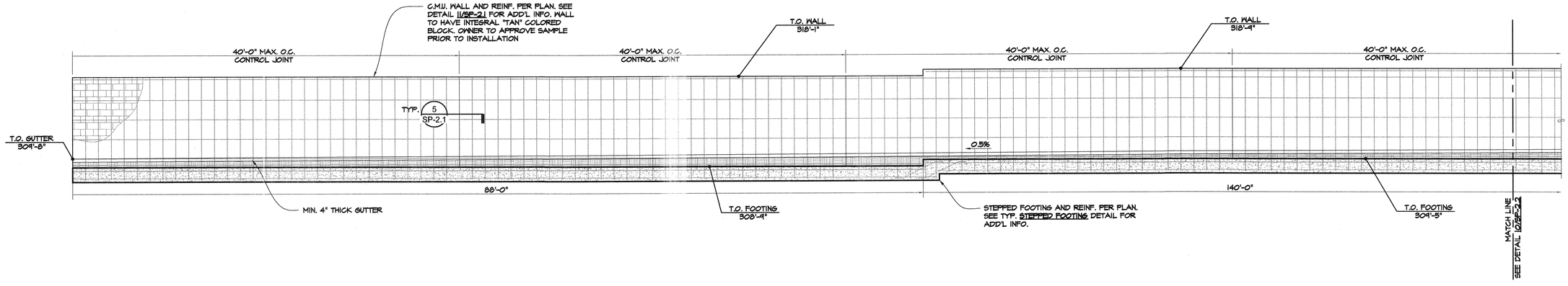
*MINIMUM REQUIRED COVER



NOTES:
1. THE SPLICE LENGTHS LISTED IN THE TABLE ABOVE ARE SHOWN IN INCHES AND REQUIRE THE FOLLOWING:
   - MASONRY COMPRESSIVE STRENGTH OF 1500 PSI
   - GRADE 60 STEEL FOR #4 AND LARGER BAR SIZES
   - GRADE 40 STEEL FOR #3 SIZE BAR
2. THE SPLICE LENGTHS LISTED IN THE TABLE ARE THE WORST CASE VALUE BETWEEN ALLOWABLE STRESS DESIGN AND STRENGTH DESIGN. THE ALLOWABLE STRESS DESIGN VALUES WERE DETERMINED IN ACCORDANCE WITH C.B.C. 2107 AND 2107A. THE STRENGTH DESIGN VALUES WERE DETERMINED IN ACCORDANCE WITH A.C.I. 530/A.S.C.E. 5/TMS 402 SECTION 3.3.3.3 AND C.B.C. 2108 AND 2108A.
3. REINFORCEMENT LARGER THAN NO. 9 SHALL BE SPLICED USING MECHANICAL CONNECTIONS IN ACCORDANCE WITH A.C.I. 530/A.S.C.E. 5/TMS 402 SECTION 2.1.10.1.9.
4. BARS SPLICED BY NON-CONTACT LAP SPLICES SHALL NOT BE SPACED FARTHER APART THAN ONE-FIFTH THE REQUIRED LENGTH OF LAP NOR MORE THAN 8"

2 LAP SPLICE SCHEDULE

FOR FULLY GROUTED CMU — SCALE: N.T.S.



12 PARTIAL ELEVATION

SCALE: 3/16" = 1'

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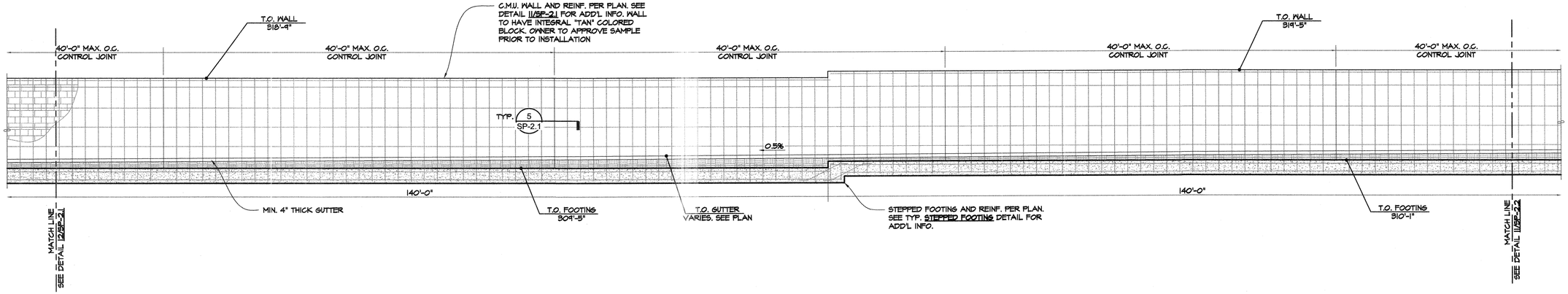
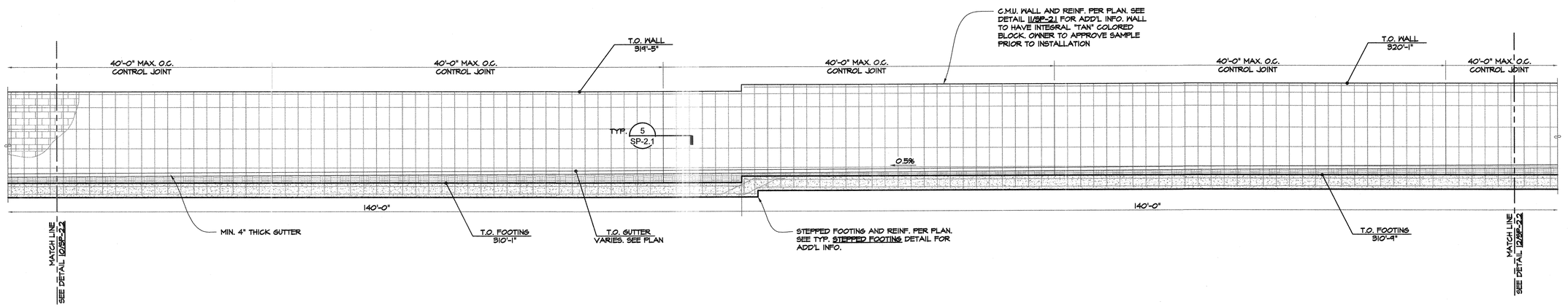
PROJECT NUMBER 19590

SHEET NUMBER SP-2.1



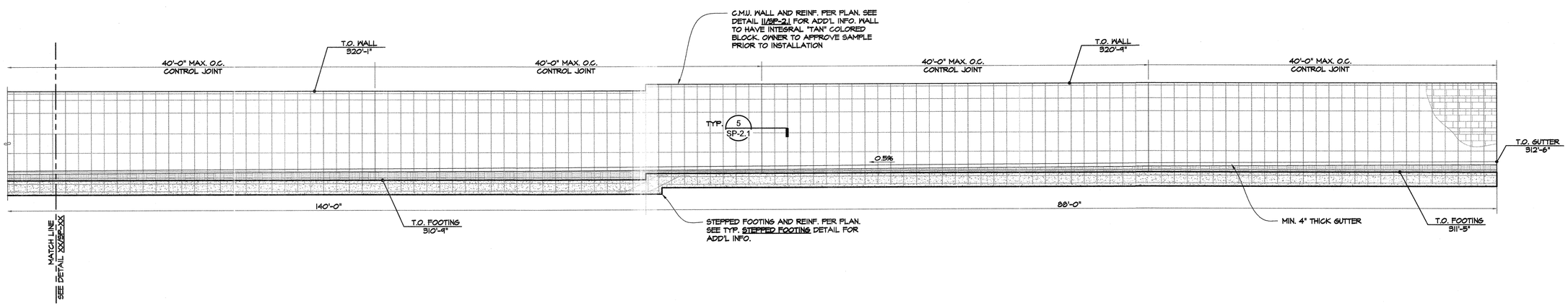
10 PARTIAL ELEVATION

SCALE: 3/16" = 1'



11 PARTIAL ELEVATION

SCALE: 3/16" = 1'



12 PARTIAL ELEVATION

SCALE: 3/16" = 1'

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02 118215
AC. TM CLS SSZ
DATE 6-29

DSA APP. 02-118215

ENGINEER	BWY
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DATE	03/06/2020
REVISIONS	

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SHEET NUMBER
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